May 30, 1907

Transportation Cabinet Department of Highways Division of Design

DESIGN EXECUTIVE SUMMARY

County	<u>FRANKLIN</u>		Item No5-056.0	00
Federa	l Project No. 0001	NH 00644 070	UPNFD43	037 0064 053-058
Project	Description: I-64	From US 127 to US 60		
Roadw Local	ay Classification:	Collector	Arterial	
	ate X	Rural X	Urban	_
	current) 29,000	ADT (2020) 52,400	DHV (2020) 5,800	
		55 (rural)	35 (urban)	
Selected Design Speed		Other (specify) 65mph 110km/h	<u> </u>	
		ace in a reduced design speed to design speed criteria will h		
Proposed 70 ft pavement 12 ft shoulders 18 ft ditches 60 ft bridges DES	IGN CRITERIA	REQUIRED	UTILIZED	EXISTING
63 ft EB (footnote) 1640 ft radius 590 ft SSD	er of Lanes Note	. 6	6	4
330.11302	ent Width	21.6m	21.6m	14.4m
Shoulder Width, Slope		3.6m @ 4%	3.6m @ 4%	3.6m @ 4%
Ditch Width, Slope		5,4m @ 1:6	5.4m @ 1:6	2.4m @ 1:4
Bridge	e Width Note :	2 18.0m ea. Direction	18.0m ea. Direction	9.1m ea. Direction
Earth	Cut Slope	1:4	1:4	1:4
Fill Slope		1:2	1:2	1:2
Minim	num Radius (cmr.= 8.09	500m	700m	700m
Maxim	num Grade	4.0%	4.0%	4.0%
Minim	num Sight Distance	180m	208m	229m

- Note 1. An additional lane is added in the eastbound direction for a truck climbing lane.
- Note 2. Existing Eastbound bridge at US 60 to be widened to 19.1 meters.

Access Control Typeruny Controlled Access				
Environmental Action	Categorical Exclusion	Approval Date	12-Aug-95	
Existing Pavement Depths	100mm DGA on Rock Subgrade with 250mm PCC Pavement	or 150mm DGA	on Soil Subgrade	
(2) Disc and	vide map showing project location cussion of all considered alternates a brief description of maintenance 2" x 11" Typical Section	s, including Do N		
Submitted By:	District Preconstruction Engineer		4/48/97 Date	
Recommended By:	Location Engineer		5 - 2 - 97	
Approved By: Acting	T. E. B. M. for Location		5 17 97 Date	
Comments:				

GEOMETRIC APPROVAL GRANTED BY:

Director Division of Design

Division Administrator Federal Highway Administration 5/12/97

Date

MAY 3 0 1997

Date



Commonwealth of Kentucky **Transportation Cabinet**

James C. Codell, III
Secretary of Transportation

Frankfort, Kentucky 40622

Paul E. Patton Governor

T. Kevin Flanery
Deputy Secretary

May 14, 1997

Mr. Jesse Story
Division Administrator
Federal Highway Administration
P. O. Box 536
Frankfort, Kentucky 40602

Dear Mr. Story:

SUBJECT:

Franklin County

000NH 00644 070

FD43 037 0064 053-058 017 D I-64 from US 127 to US 60

Item No. 5-56.00

Transmitted are the original and one copy of the Design Executive Summary for your review and approval of the geometrics for this project. Please return the original to this office.

BY:

Sincerely yours,

J. M. Yowell, P.E.

State Highway Engineer

John Sacksteder, P.E.,

Director, Division of Highway Design

John B. Sacksledn

Attachment:

cc: Charles Raymer FHWA File

MAY 1 8 1997

To Int.

DA
ADA
HPG
HPJ
BR
R/W
ENV
HPT
AM
FIS

STATE

RECEIVED

Ky. Division

DESIGN EXECUTIVE SUMMARY NARRATIVE

This section of I-64 in Franklin County lies in a rural area with severely rolling terrain between the interchange with US 127 on the west and the interchange with US 60 on the east. The alternatives for this section were studied within the boundaries of the existing right of way. This section has a constant-width median at each end of the project with bifurcated roadways in the middle with separation ranging from 55 meters to as much as 152 meters. Within this section, there are bridges over Cedar Run Creek at KY 420, Johnson Road and the Kentucky River and Glenns Creek Road. The bridge at Hanley Lane is over I-64 and bridge widening is required at I-64 eastbound over US 60.

180-500 ft

Do-Nothing Alternate

The existing section consists of two - 24 foot roadways with 3 foot paved shoulders on the left side and 10 foot paved shoulders on the right side of each roadway, and eight foot ditches. This section is also through deep rock cuts on both sides that were not pre-split This typical section does not meet current design criteria for 110 70 when constructed. km/h roadways with required shoulder widths and clear zones. The existing bridges do not provide full-width shoulders. This alternate does not provide three lanes in each direction and a truck climbing lane in the eastbound roadway to meet current or future traffic forecasts in the capacity analysis for these roadways. These conditions combine to form an undesirable existing facility.

Alternate No. 1

This alternate would provide a new six-lane roadway with a uniform median barrier throughout the project limits with a truck climbing lane in the eastbound roadway, east of the Kentucky River to approximately 400 meters east of the Hanley Lane bridge. 1300 ft Complete new structures would be needed at all four bridge locations; KY 420, Johnson Road, Kentucky River, and Hanley Lane. The vertical alignment on this alternate is similar to the grades on the existing roadways. This alternate would require more excavation and leave more waste material than all other Alternates except No. 1A. Maintenance of traffic could be accommodated on the existing roadways, except at the project termini where some construction staging would be required in order to maintain two lanes of traffic in each direction.

Alternate No. 1A

This alternate is basically the same as alternate No. 1, however, it utilizes an 18.3 meter (60 foot) depressed median throughout the current bifurcated section. It would require more excavation and waste material than Alternate 1.

Alternate No. 2

This alternate widened the existing roadway to provide an additional lane in each direction. The widening requires that the existing horizontal and vertical geometry would be retained. The alignment would be shifted to provide for maintenance of traffic and a fall bench in the existing rock cuts. Widening or replacing the bridges is required with this alternate.

Alternate No. 3

This alternate is a mixture of Alternates 1 and 2. The westbound roadway would be as discussed with Alternate No. 2. The existing roadway would be widened and shifted to provide for maintenance of traffic and a fall bench. The eastbound roadway would be similar to Alternate No. 1. It would be separated from the existing eastbound roadway and flattened to improve the grade up from the Kentucky River. The eastbound alignment would require new bridge construction.

Preferred Alternate

This alternate is a product of the study of the preceding alternates and contains elements of each. This alternate consists of reconstructing the roadways to provide a third lane on the inside of the existing roadways in both eastbound and westbound directions and the addition of an eastbound truck climbing lane up the hill from the Kentucky River. The vertical alignment on this alternate is similar to the grades on the existing roadways except that the grade of the eastbound roadway across the Kentucky River approximately to Hanley Lane will be flattened from a 3.78% grade to a 3.52% grade. The flattening of this grade improves operation and reduces the length of the Truck Climbing Lane for this section of the eastbound roadway.

The existing rock cut slopes on this portion of I-64 were not presplit and create continuous ditch maintenance from rock fall. A 5.5 meter fall bench, between the existing rock face and the edge of the clear zone, was recommended by the geotechnical division. This 5.5 meter offset located the horizontal alignments so that widening the existing bridges is impractical. The alignments were then shifted further to facilitate bridge footing construction.

The project team, at the FHWA's request, addressed the possibility of widening the existing bridges and roadways. The alignments would have to be located at or near their current location to facilitate the widening. This alternative would not allow sufficient room for the fall bench, requiring presplitting of the existing rock slopes and additional right of way along I-64. The alignment would increase the difficulty and expense of maintaining traffic.

This section of Interstate is approximately 35 years old and the bridges have utilized a significant portion of their useful life and become outdated due to changes in design criteria for loading conditions. The Kentucky River bridges, the most expensive on the project, are non-redundant load path structures. A bridge inspection report from 1988 indicated that in general, "the quality of all welds on the structures were very poor, and that some cracks were found in construction welds between the tips of the horizontal and vertical stiffeners. A lack of fusion, undercutting, and pitting of welds was found at several locations." The cost to replace these bridges is estimated at \$18,847,371. The cost to widen these bridges is estimated at \$17,523,157. The difference in costs is \$1,324,214 which is approximately two percent of the total project cost. Widening these bridges does not increase the life span of the bridges nor does it correct the non-redundant load path of the structure. The total amount for widening these bridges does not include the additional cost of right of way or blasting excavation to correct the outside cut slopes throughout the project. The existing bridges were built in 1963 with HS20 design standards which does not meet current HS25 design standards. After considerable discussion, the project team decided not to pursue this alternative.

US 127 Interchange

2950 ft

The project would begin approximately 900 meters west of the terminus of the westbound ramp from US 127 to I-64. This is the point where the 3 lane westbound section would complete the taper to the existing two lane section. The eastbound added lane would begin near the intersection with US 127, and would require the eastbound ramp terminal from US 127 to I-64 to be realigned to meet the lane shift of the eastbound roadway of I-64.

US 60 Interchange

4200 ft

The project will terminate approximately 1290 meters east of the existing eastbound ramp gore area from US 60 to I-64. This is the point where the 3 lane eastbound section would complete the taper to the existing two lane section. The existing ramp terminal does not meet the current design standards for this classification of road and would be reconstructed. The taper for the westbound added lane would begin at the west end of the westbound bridge over US 60. The eastbound I-64 to U.S. 60 ramp would be widened to provide dual left turns.

Conceptual Maintenance of Traffic

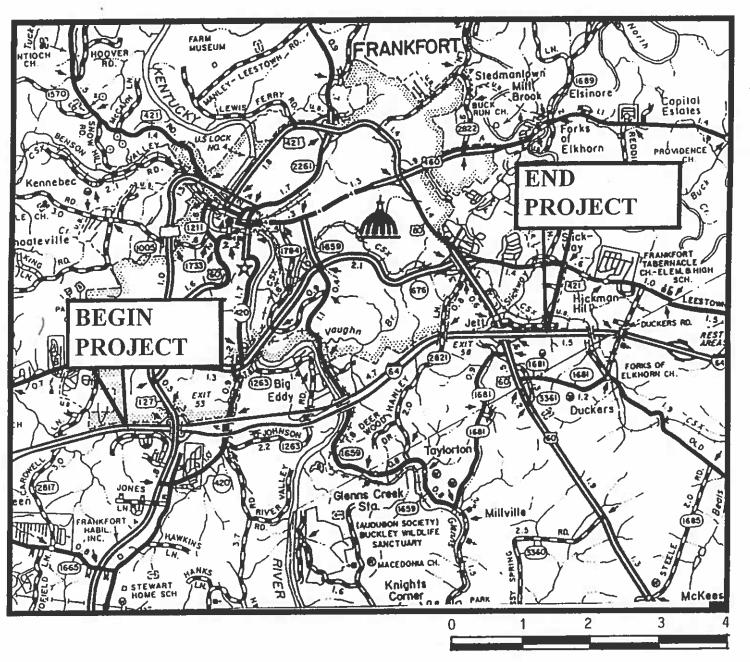
The bifurcated alignments were shifted toward the median sufficiently to maintain two lanes of traffic on the existing roadways while constructing part-width ultimate roadways.

The original phasing plan would have shifted traffic to the new part-width roadways while completing the facilities.

After studying several maintenance of traffic options, however, the most favorable is to build the part-width westbound lanes while maintaining traffic on the existing roadways. Eastbound traffic would then be shifted to the part-width westbound lanes using crossovers at each end of the project. Temporary concrete barrier walls would be required due to the proximity of the opposing eastbound and westbound traffic. Some temporary drainage structures would be required during construction to prevent ponding between the roadways.

With all traffic on the westbound side of the bifurcated roadway, the entire eastbound roadway can be reconstructed without affecting traffic. This method will also allow for the replacement of any drainage structures required for the eastbound roadway. It also eliminates some constructability concerns associated with the flattening of the eastbound grade up from the Kentucky River.

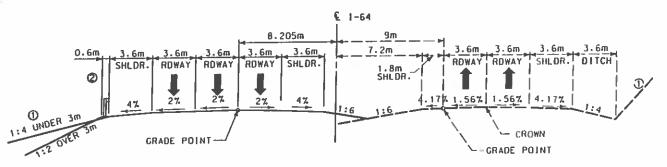
At the existing depressed median locations (each end of the project) traffic can be moved to the new inside shoulder and lane while the two outside lanes are being reconstructed. Some additional part-width construction is required at ramps and ramp terminals. Temporary barrier walls will be used to separate the traffic from construction activities.



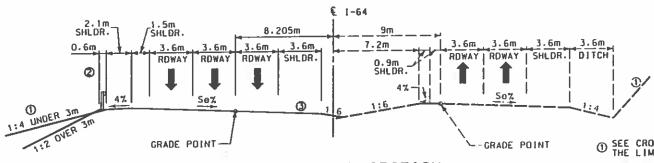
FRANKLIN

TYPICAL SECTIONS

STA. 99+160 TO STA. 101+025.860 BK.



NORMAL SECTION
WITH DEPRESSED MEDIAN

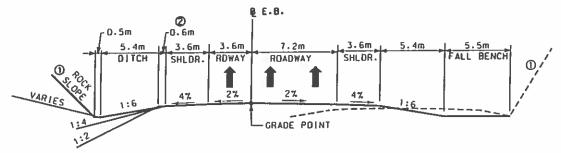


SUPERELEVATED SECTION WITH DEPRESSED MEDIAN

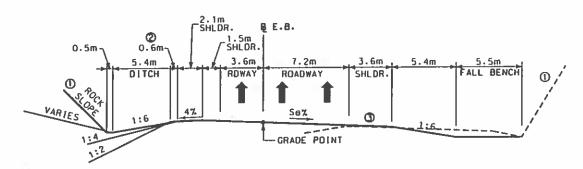
- ① SEE CROSS SECTIONS FOR SLOPES OUTSIDE THE LIMITS OF THE SHOULDERS.
- ② SHOULDERS SHALL BE WIDENED .6m WHERE GUARDRAIL IS REQUIRED.
- SUPERELEVATED SHOULDERS SHALL BE CONSTRUCTED TO STANDARD SUPERELEVATION EXCEPT NOT FLATTER THAN 4.0%.

EASTBOUND LANES

STA. 101+025.860 TO STA. 104+936.647 STA. 107+730.000 TO STA. 107+846.939



Normal Section Eastbound Lone without Truck Climbing Lane



Superelevated Section Eastbound Lane without Truck Climbing Lane

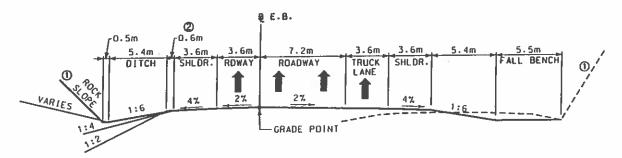
- ① SEE CROSS SECTIONS FOR SLOPES OUTSIDE THE LIMITS OF THE SHOULDERS.
- ② SHOULDERS SHALL BE WIDENED .6m WHERE GUARDRAIL IS REQUIRED.
- 3 SUPERELEVATED SHOULDERS SHALL WE CONSTRUCTED TO STANDARD SUPERELEVATION EXCEPT NOT FLATTER THAN 4.0%.

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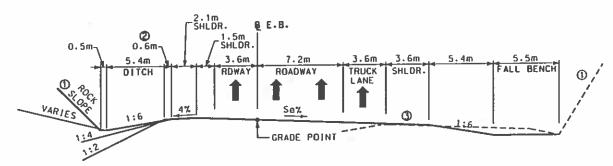
TYPICAL SECTIONS

EASTBOUND LANES

STA. 104+936.647 TO STA. 107+730



Normal Section Eastbound Lane with Truck Climbing Lane



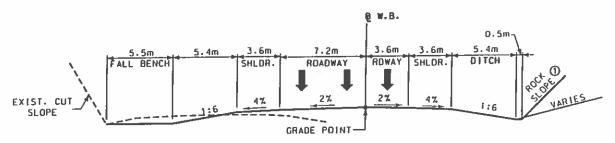
Superelevated Section Eastbound Lane with Truck Climbing Lane

- \odot SEE CROSS SECTIONS FOR SLOPES OUTSIDE THE LIMITS OF THE SHOULDERS.
- SHOULDERS SHALL BE WIDENED .6m WHERE GUARDRAIL IS REQUIRED.
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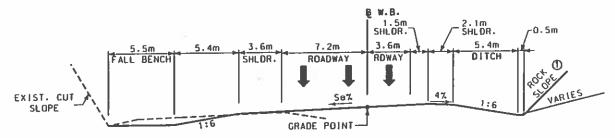
COLUMN	5.64	PART	973			
COVERT 10						
Jam. So., 5-654-20						

WESTBOUND LANES

STA. 101+025.869 TO STA. 107+804.901



Normal Section Westbound Lane



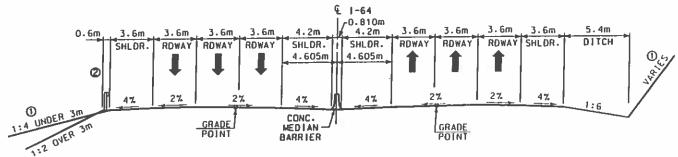
Superelevated Section Westbound Lone

- ① SEE CROSS SECTIONS FOR SLOPES OUTSIDE THE LIMITS OF THE SHOULDERS.
- SHOULDERS SHALL BE WIDENED .6m WHERE GUARDRAIL IS REQUIRED.
- SUPERELEVATED SHOULDERS SHALL BE CONSTRUCTED TO STANDARD SUPERELEVATION EXCEPT NOT FLATTER THAN 4.0%.

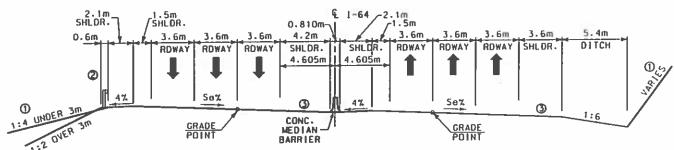
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STA. 107+846.939 TO STA. 108+570



NORMAL SECTION WITH MEDIAN BARRIER

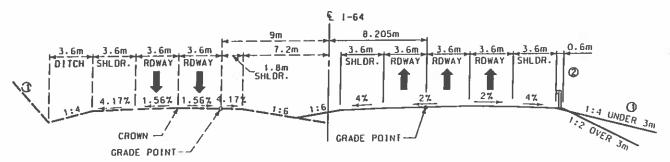


SUPERELEVATED SECTION WITH MEDIAN BARRIER

- Φ see cross sections for slopes outside the limits of the shoulders.
- ② SHOULDERS SHALL BE WIDENED .6m WHERE GUARDRAIL IS REQUIRED.
- 3 SUPERELEVATED SHOULDERS SHALL BE CONSTRUCTED TO STANDARD SUPERELEVATION EXCEPT NOT FLATTER THAN 4.0%.

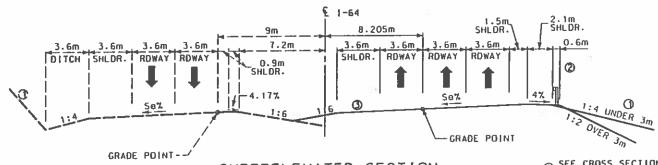
FRANKLIN WALL STATE

STA. 108+675 TO STA. 110+320



NORMAL SECTION
WITH DEPRESSED MEDIAN

SEE DETAIL SHEET FOR SPECIAL TRANSITION FROM STA. ___+_ TO STA. ___+__



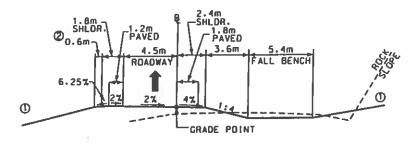
SUPERELEVATED SECTION WITH DEPRESSED MEDIAN

- ① SEE CROSS SECTIONS FOR SLOPES OUTSIDE THE LIMITS OF THE SHOULDERS.
- ② SHOULDERS SHALL BE WIDENED .6m WHERE GUARDRAIL IS REQUIRED.
- 3 SUPERELEVATED SHOULDERS SHALL BE CONSTRUCTED TO STANDARD SUPERELEVATION EXCEPT NOT FLATTER THAN 4.0%.

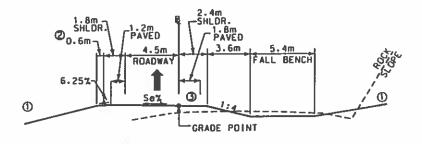
Lawer to

TYPICAL SECTIONS

U.S. 127 RAMPS



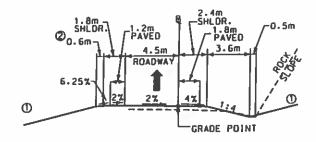
Normal Section U.S. 127 RAMP



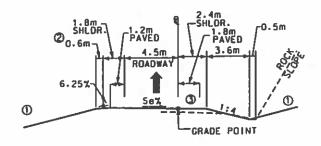
Superelevated Section U.S. 127 RAMP

- ① SEE CROSS SECTIONS FOR SLOPES OUTSIDE THE LIMITS OF THE SHOULDERS.
- © SHOULDERS SHALL BE WIDENED .6m WHERE GUARDRAIL IS REQUIRED.
- 3 SUPERELEVATED SHOULDERS SHALL BE CONSTRUCTED TO STANDARD SUPERELEVATION EXCEPT NOT FLATTER THAN 4.0%.

U.S. 60 RAMPS



Normal Section U.S. 60 RAMP

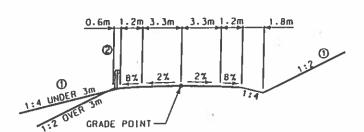


Superelevated Section U.S. 60 RAMP

- $\ensuremath{\mathfrak{D}}$ SEE cross sections for slopes outside the limits of the shoulders.
- ② SHOULDERS SHALL BE WIDENED .6m WHERE GUARDRAIL IS REQUIRED.
- 3 SUPERELEVATED SHOULDERS SHALL BE CONSTRUCTED TO STANDARD SUPERELEVATION EXCEPT NOT FLATTER THAN 4.0%.

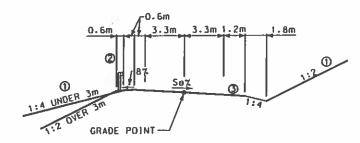
HANLEY LANE





NORMAL SECTION

- \odot SEE CROSS SECTIONS FOR SLOPES OUTSIDE THE LIMITS OF THE SHOULDERS.
- © SHOULDERS SHALL BE WIDENED .6m WHERE GUARDRAIL IS REQUIRED.
- 3 SUPERELEVATED SHOULDERS SHALL BE CONSTRUCTED TO STANDARD SUPERELEVATION EXCEPT NOT FLATTER THAN 8.0%.



SUPERELEVATED SECTION

